



Universidad
Carlos III de Madrid
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**Department of Continuum Mechanics and Structural
Engineering**

Aerospace Structures

Chapter 4. Plates and Shells **Shells of revolution**



Chapter 4. Plates and Shells

Shells of revolution

1. Introduction
2. Membrane theory
3. Basic geometrical relations
4. Equilibrium equations for shells of revolution
5. Displacement equations for shells of revolution
6. References

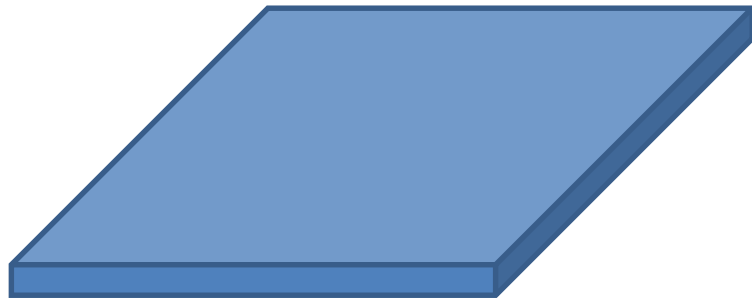


Chapter 4. Plates and Shells

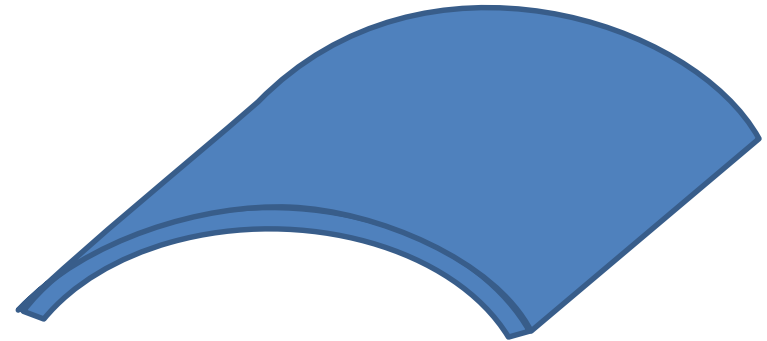
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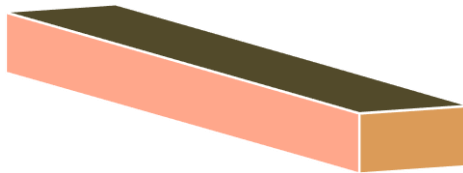
Differences between plates and shells?



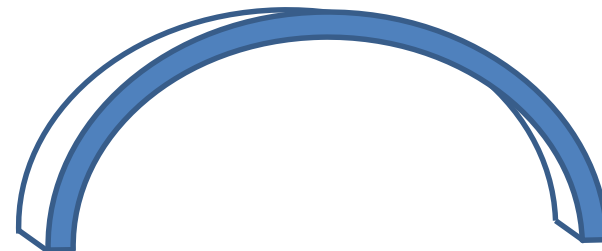
Plate



Shell



Beam



Arch

Differences between beams and arches?



General definition

A structural membrane or shell is a curved surface structure. Usually, it is capable of transmitting loads in more than two directions to supports. It is highly efficient structurally when it is so shaped, proportioned, and supported that it transmits the loads without bending or twisting.

A membrane or a shell is defined by its middle surface, halfway between its extrados, or outer surface and intrados, or inner surface. Thus, depending on the geometry of the middle surface, it might be a type of dome, barrel arch, cone, or hyperbolic paraboloid. Its thickness is the distance, normal to the middle surface, between extrados and intrados.



Advantages of shells structures

1. Efficiency of load-carrying behaviour
2. High degree of reserved strength and structural integrity
3. High specific strength (strength/ weight ratio)
4. Very high stiffness
5. Containment of space



Thin shells

A thin shell is a shell with a thickness relatively small compared with its other dimensions. But it should not be so thin that deformations would be large compared with the thickness.

Calculation of the stresses in a thin shell generally is carried out in two major steps, both usually involving the solution of differential equations. In the first, bending and torsion are neglected (**membrane theory**). In the second step, corrections are made to the previous solution by superimposing the bending and shear stresses that are necessary to satisfy boundary conditions (**bending theory**)



Examples of thin shells

- Civil and architectural engineering
 - ✓ Large span-roofs
 - ✓ Liquid-retaining structures and water tanks
 - ✓ Concrete arch domes
- Mechanical engineering
 - ✓ Piping systems
 - ✓ Turbine disks
 - ✓ Pressure vessels
- Biomechanics

AEROSPACE STRUCTURES



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Kirchhoff-Love hypothesis

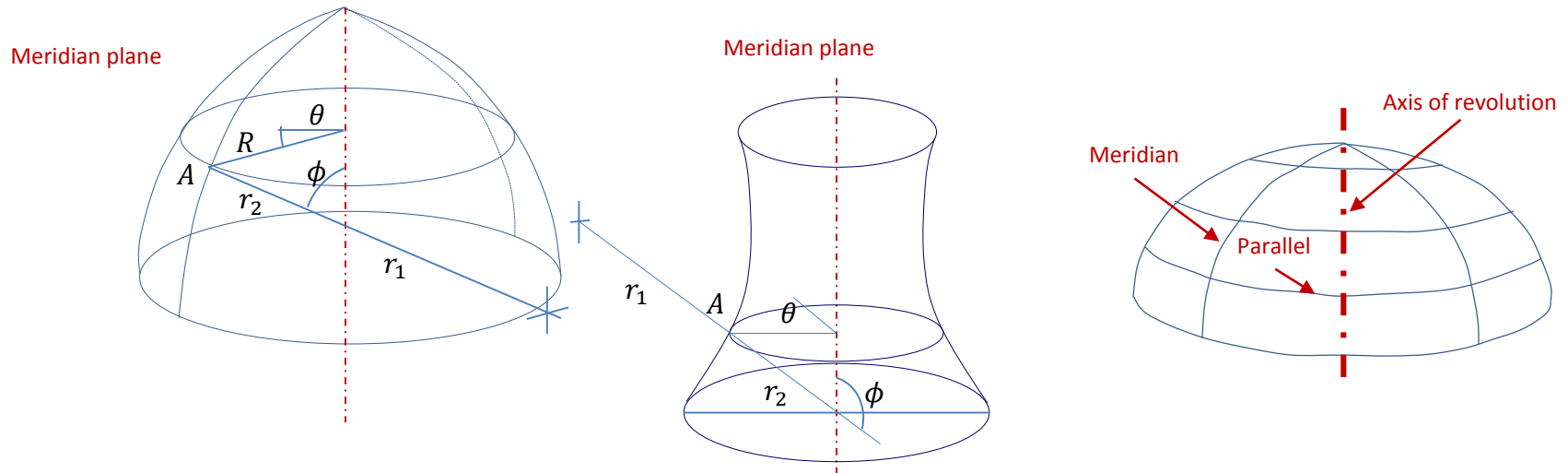
- Thickness is small comparing to the curvature radius of the mid-surface
- Small displacements and strains (equilibrium is verified in the undeformed shape)
- Straight lines normal to the mid-surface remain straight and normal after deformation
- Stresses in perpendicular direction to the mid-surface are neglected



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ϕ : meridian angle

r_1 : first main radius. It is the curvature radius of the meridian

θ : circumferential angle

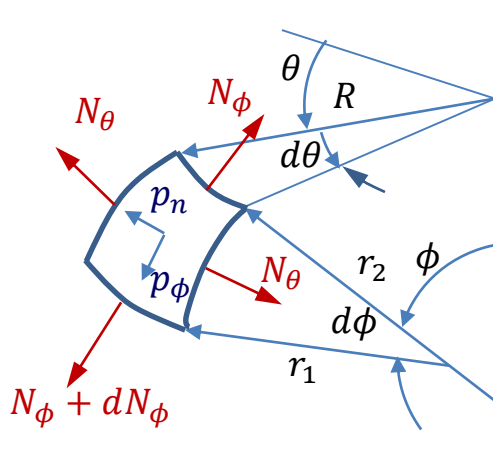
r_2 : second main radius. It is the distance to the revolution axis in normal direction to the shell surface



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- Bending and torsion moments are neglected (Membrane theory)
- Shear forces are neglected in mid-surface (Axial-symmetry)

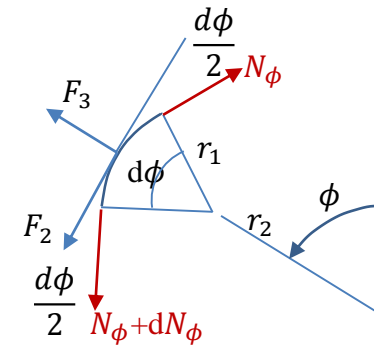
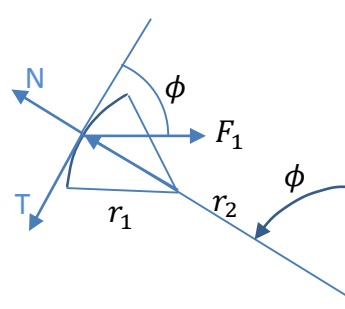
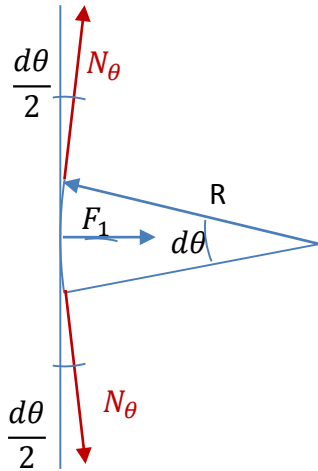
Internal forces:

- N_ϕ : per unit length in meridian direction
- N_θ : per unit length in circumferential direction (independent on θ)

External loads per unit surface

- p_n : in normal direction
- p_ϕ : in meridian direction
- p_θ : does not exist (axial-symmetrical load)

Equilibrium in meridian direction



$$F_1 = 2N_\theta r_1 d\phi \operatorname{sen} \frac{d\theta}{2} \approx N_\theta r_1 d\phi d\theta$$

Tangential component

$$-F_1 \cos \phi = -N_\theta r_1 \cos \phi d\phi d\theta$$

$$F_2 \approx N_\phi dR d\theta + R dN_\phi d\theta$$

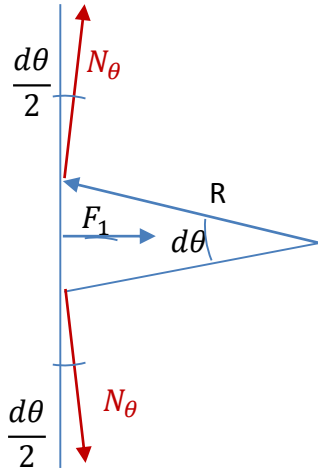
$$F_2 \approx d(RN_\phi) d\theta$$

Tangential component
of the external load:

$$p_\phi r_1 R d\phi d\theta$$

$$d(RN_\phi) - N_\theta r_1 \cos \phi d\phi + p_\phi r_1 R d\phi = 0$$

Equilibrium in normal direction



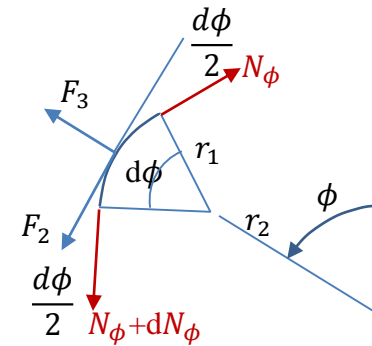
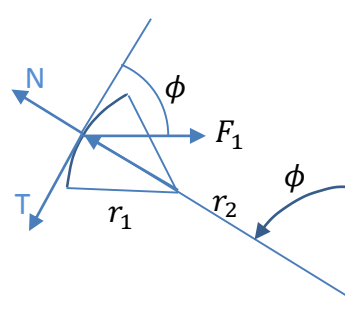
$$F_1 \approx N_\theta r_1 d\phi d\theta$$

Normal component:

$$-F_1 \sin\phi = -N_\theta r_1 \sin\phi d\phi d\theta$$

Normal component
of the external load:

$$p_n r_1 R d\phi d\theta$$



$$F_3 = -N_\phi R d\theta \sin\frac{d\phi}{2} - (N_\phi + dN_\phi)(R + dr) d\theta \sin\frac{d\phi}{2}$$

$$F_3 \approx -RN_\phi d\theta d\phi$$

$$\frac{N_\phi}{r_1} + \frac{N_\theta}{r_2} = p_n$$

$$d(RN_\phi) - N_\theta r_1 \cos \phi d\phi + p_\phi r_1 R d\phi = 0$$

$$\frac{N_\phi}{r_1} + \frac{N_\theta}{r_2} = p_n$$

$$d(RN_\phi) - r_1 \left(r_2 p_n - \frac{r_2}{r_1} N_\phi \right) \cos \phi d\phi = -r_1 R p_\phi d\phi$$

Multiplying by $\frac{\sin \phi}{d\phi}$ and considering $R = r_2 \sin \phi$

$$\frac{d(r_2 \sin^2 \phi N_\phi)}{d\phi} = r_1 r_2 p_n \cos \phi \sin \phi - r_1 r_2 \sin^2 \phi p_\phi$$

Integrating over ϕ

$$N_\phi = \frac{\int r_1 r_2 \sin \phi (p_n \cos \phi - p_\phi \sin \phi) d\phi + k}{r_2 \sin^2 \phi}$$

k is obtained from
boundary conditions



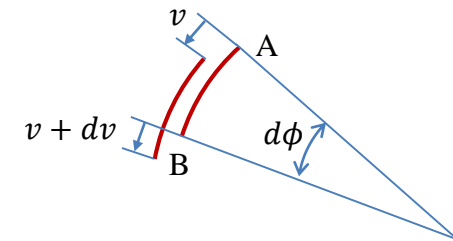
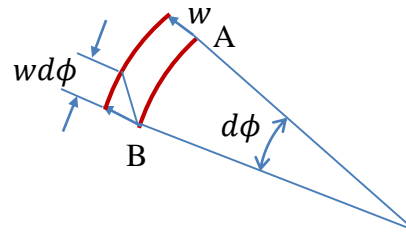
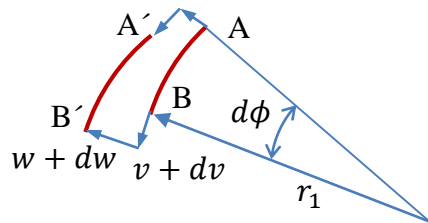
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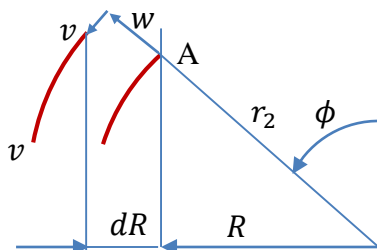
Displacements: v , tangent to meridian; w , in normal direction

Strains



$$\varepsilon_\phi = \frac{A'B' - AB}{AB} = \frac{dv + wd\phi}{r_1 d\phi} = \frac{1}{r_1} \left(\frac{dv}{d\phi} + w \right)$$

v



$$\varepsilon_\theta = \frac{2\pi(R + dR) - 2\pi R}{2\pi R} = \frac{dR}{R} \quad dR = v \cos \phi + w \text{sen} \phi$$

$$\varepsilon_\theta = \frac{v \cos \phi + w \text{sen} \phi}{R} = \frac{1}{r_2} (v \cot \phi + w)$$

with $R = r_2 \text{sen} \phi$

Strains

$$\frac{dv}{d\phi} + w = r_1 \varepsilon_\phi$$

$$v \cot \phi + w = r_2 \varepsilon_\theta$$

Hook laws

$$\varepsilon_\phi = \frac{1}{E} (\sigma_\phi - \nu \sigma_\theta) = \frac{1}{Eh} (N_\phi - \nu N_\theta)$$

$$\varepsilon_\theta = \frac{1}{E} (\sigma_\theta - \nu \sigma_\phi) = \frac{1}{Eh} (N_\theta - \nu N_\phi)$$

$$\frac{dv}{d\phi} - v \cot \phi = \frac{1}{Eh} [(r_1 + \nu r_2) N_\phi - (r_2 + \nu r_1) N_\theta]$$

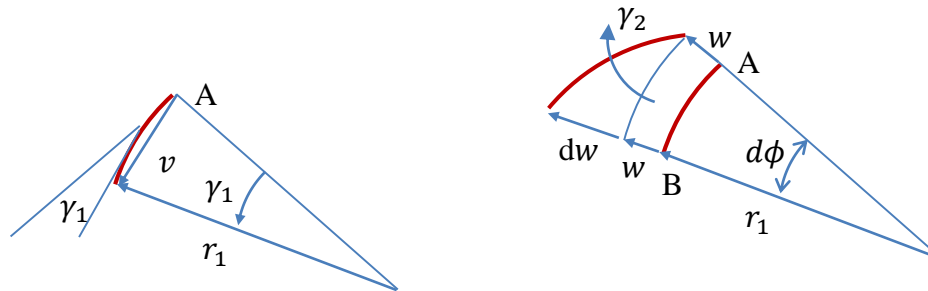
$$\text{sen} \phi \frac{d}{d\phi} \left(\frac{v}{\text{sen} \phi} \right)$$

$$v = \text{sen} \phi \left\{ \frac{1}{Eh} \int \frac{1}{\text{sen} \phi} [(r_1 + \nu r_2) N_\phi - (r_2 + \nu r_1) N_\theta] d\phi + k \right\}$$

$$w = r_2 \varepsilon_\theta - v \cot \phi = \frac{r_2}{Eh} (N_\theta - \nu N_\phi) - v \cot \phi$$

k is obtained from
boundary conditions

Meridian rotation



γ_1 : rotation motivated by the displacement v of point A

γ_2 : rotation motivated by the difference between displacements w of points A and B

γ : meridian rotation

$$\gamma = \gamma_1 - \gamma_2 = \frac{v}{r_1} - \frac{dw}{r_1 d\phi}$$

δ : horizontal displacement

$$\delta = R\varepsilon_\theta = \frac{1}{Eh} r_2 \text{sen}\phi (N_\theta - \nu N_\phi)$$

Meridian rotation

$$\gamma = \gamma_1 - \gamma_2 = \frac{v}{r_1} - \frac{dw}{r_1 d\phi}$$

$$\frac{d}{d\phi}(v \cot \phi + w) = \frac{dv}{d\phi} \cot \phi - \frac{v}{\sin^2 \phi} = r_2 \frac{d\varepsilon_\theta}{d\phi}$$

On the other hand

$$\left(\frac{dv}{d\phi} - v \cot \phi \right) \cot \phi = \frac{dv}{d\phi} \cot \phi - v \frac{\cos^2 \phi}{\sin^2 \phi} = \frac{\cot \phi}{Eh} \left[(r_1 + \nu r_2) N_\phi - (r_2 + \nu r_1) N_\theta \right]$$

Subtracting the above equations and dividing by r_1

$$\gamma = \frac{1}{r_1} \left\{ \frac{\cot \phi}{Eh} \left[(r_1 + \nu r_2) N_\phi - (r_2 + \nu r_1) N_\theta \right] - \frac{d}{d\phi} \left[\frac{1}{Eh} (N_\theta - \nu N_\phi) \right] \right\}$$



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1. **J. A. Jurado Albarracín-Martinón y S. Hernández Ibáñez, “Análisis estructural de placas y láminas”. Tercera Edición. Andavira editora 2014**
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3. **Ugural, A.C. “Stresses in beams, plates and shells”. CRC. Taylor & Francis, 2009**